

LOAD CAPACITY

Wood screw ESSDRIVE countersunk head 5.0 – 6.0 mm. Electro zinc plated

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Conditions for tabulated load capacity

The tabulated values are calculated in accordance with Eurocode 5 (EN 1995-1-1:2004 incl. AC:2006, A1:2008 and A2:2014). The calculation assumes that the entire threaded part B is screwed into the underlying timber part and that it has at minimum the same thickness, i.e. $t_2 \geq B$. Furthermore it is assumed the two timber parts are made of the same timber quality class (e.g. C24). If the screw is subjected to both axial and shear load the total load capacity must be verified. The tabulated loads are for one screw, if more screws are used a reduction may be needed depending on spacing. The final design should consider edge and spacing distances.

Recommended load

The recommended load given in unit [kg] can be applied directly since all safety factors have been considered incl. a factor on the applied load ($\gamma = 1.4$). It is calculated for a permanent load and service class 2 (acc. to Eurocode 5).

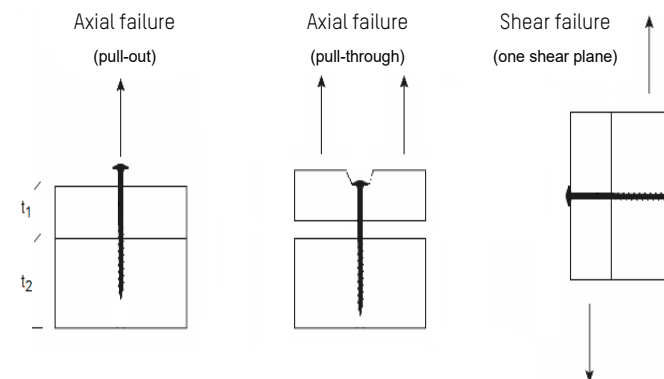
Characteristic resistance

The characteristic resistance given in unit [kN] is intended for an engineer that wants to do a detailed analysis of the timber connection using the appropriate partial coefficients for design resistance based on load duration and service class in accordance with Eurocode 5 eq. (2.17):

$$R_d = k_{mod} \frac{R_k}{\gamma_M}$$

All information in this document is given in accordance with known facts and information at the time of writing. The information is subject to change without further notification. The document is updated continuously in conjunction with regular revision or in the event of major-specific technical changes.

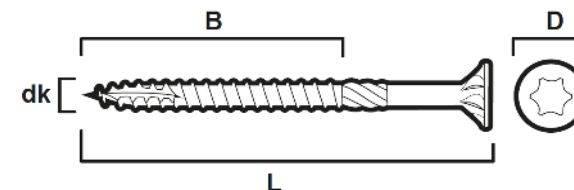
All advice given by ESSVE should only be seen as guidance and does not mean that ESSVE can be held responsible for the advice provided. It is always the customer's own responsibility to decide on the choice of product, usage, application, etc. The supplier's advice is only a part of the customer's basis for decision making.



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Recommended load

Intended for craftsmen

Art. No.	CE-marking EN 14592	Dimension dk × L [mm]	Thread length B [mm]	Inner thread diameter d ₁ [mm]	Head diameter D [mm]	Timber tickness at screw head t ₁ [mm]	Timber tickness at screw tip t ₂ [mm]	Axial direction (pull-out/-through) F _{ax,rec} [kg]		Shear direction (one shear plane) F _{v,rec} [kg]	
								C14	C24	C14	C24
136 156	✓	5.0×25	20	3.0	10	5	20	50	55	5	10
136 125	✓	5.0×30	20	3.0	10	10	20	50	55	15	20
136 157	✓	5.0×35	24	3.0	10	11	24	60	65	20	20
136 126	✓	5.0×40	24	3.0	10	16	24	60	65	25	35
136 127	✓	5.0×50	30	3.0	10	20	30	60	70	30	35
136 128	✓	5.0×60	36	3.0	10	24	36	60	70	35	40
136 129	✓	5.0×70	42	3.0	10	28	42	60	70	35	40
136 130	✓	5.0×80	42	3.0	10	38	42	60	70	40	45
136 131	✓	5.0×90	42	3.0	10	48	42	60	70	40	45
136 132	✓	5.0×100	52	3.0	10	48	52	60	70	40	45
136 133	✓	5.0×120	62	3.0	10	58	62	60	70	40	45

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Recommended load

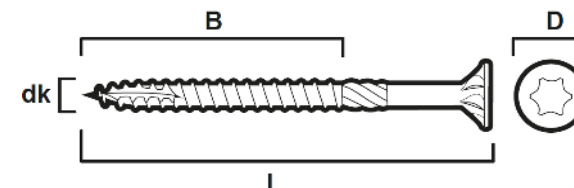
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Art. No.	CE-marking EN 14592	Dimension dk × L [mm]	Thread length B [mm]	Inner thread diameter d ₁ [mm]	Head diameter D [mm]	Timber tickness at screw head t ₁ [mm]	Timber tickness at screw tip t ₂ [mm]	Axial direction (pull-out/-through) F _{ax,rec} [kg]		Shear direction (one shear plane) F _{v,rec} [kg]	
								C14	C24	C14	C24
136 159	✓	6.0×40	24	3.6	12	16	24	60	75	30	40
136 140	✓	6.0×50	30	3.6	12	20	30	80	90	40	50
136 141	✓	6.0×60	36	3.6	12	24	36	90	105	45	55
136 142	✓	6.0×70	42	3.6	12	28	42	90	105	50	60
136 143	✓	6.0×80	42	3.6	12	38	42	90	105	55	65
136 160	✓	6.0×90	42	3.6	12	48	42	90	105	60	70
136 144	✓	6.0×100	52	3.6	12	48	52	90	105	60	70
136 145	✓	6.0×110	62	3.6	12	48	62	90	105	60	70
136 146	✓	6.0×120	72	3.6	12	48	72	90	105	60	70
136 147	✓	6.0×130	72	3.6	12	58	72	90	105	60	70
136 148	✓	6.0×140	72	3.6	12	68	72	90	105	60	70
136 149	✓	6.0×150	72	3.6	12	78	72	90	105	60	70

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Characteristic resistance

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								C14	C24	C14	C24
136 156	✓	5.0×25	20	3.0	10	5	20	1.5	1.7	0.3	0.3
136 125	✓	5.0×30	20	3.0	10	10	20	1.5	1.7	0.5	0.7
136 157	✓	5.0×35	24	3.0	10	11	24	1.8	2.1	0.6	0.7
136 126	✓	5.0×40	24	3.0	10	16	24	1.8	2.1	0.9	1.1
136 127	✓	5.0×50	30	3.0	10	20	30	1.9	2.2	1.0	1.2
136 128	✓	5.0×60	36	3.0	10	24	36	1.9	2.2	1.1	1.3
136 129	✓	5.0×70	42	3.0	10	28	42	1.9	2.2	1.1	1.3
136 130	✓	5.0×80	42	3.0	10	38	42	1.9	2.2	1.2	1.3
136 131	✓	5.0×90	42	3.0	10	48	42	1.9	2.2	1.2	1.3
136 132	✓	5.0×100	52	3.0	10	48	52	1.9	2.2	1.2	1.3
136 133	✓	5.0×120	62	3.0	10	58	62	1.9	2.2	1.2	1.3

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								C14	C24	C14	C24
136 159	✓	6.0×40	24	3.6	12	16	24	1.9	2.2	1.0	1.2
136 140	✓	6.0×50	30	3.6	12	20	30	2.4	2.8	1.2	1.5
136 141	✓	6.0×60	36	3.6	12	24	36	2.7	3.2	1.5	1.8
136 142	✓	6.0×70	42	3.6	12	28	42	2.7	3.2	1.6	1.8
136 143	✓	6.0×80	42	3.6	12	38	42	2.7	3.2	1.7	2.0
136 160	✓	6.0×90	42	3.6	12	48	42	2.7	3.2	1.8	2.1
136 144	✓	6.0×100	52	3.6	12	48	52	2.7	3.2	1.9	2.1
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136 146	✓	6.0×120	72	3.6	12	48	72	2.7	3.2	1.9	2.1
136 147	✓	6.0×130	72	3.6	12	58	72	2.7	3.2	1.9	2.1
136 148	✓	6.0×140	72	3.6	12	68	72	2.7	3.2	1.9	2.1
136 149	✓	6.0×150	72	3.6	12	78	72	2.7	3.2	1.9	2.1

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Conversion factors for load-duration and service classes

The conversion factors can be used to re-calculated the recommended load in the tables for other load-durations and service classes. The conversion factors are based on the factor k_{mod} in Eurocode 5.

Load-duration classes can differ between different countries due to climate-based loads (snow, wind).

Conversion factors from permanent load duration in service class 2

Load-duration	Examples of loading	Service class 1-2	Service class 3
Permanent	Self-weight	1.00	0.83
Long-term	Storage	1.17	0.92
Medium-term	Imposed floor load, snow	1.33	1.08
Short-term	Snow, wind	1.50	1.17
Instantaneous	Wind, accidental load	1.83	1.50

Corrosion protection

Rules and best practice for corrosion protection may differ among European countries. The end-user should ensure that the corrosion protection is suitable for the current application.

Conversion to different timber quality

Re-calculation of load capacity in the axial direction for a different timber quality (characteristic density) is possible according to the formula below:

$$F_{ax(\rho_{k,1})} \times \left(\frac{\rho_{k,2}}{\rho_{k,1}} \right)^{0,8} = F_{ax(\rho_{k,2})}$$

If for example the load capacity in axial direction is 60 kg in C14-timber the load capacity in C35-timber is increased to:

$$60kg \times \left(\frac{400}{290} \right)^{0,8} = 75kg$$

Re-calculation for load capacity in the shear direction in the same way is however not possible. For guidance please contact ESSVE technical support.

Material	Density
	ρ_k [kg/m ³]
C14	290
C18	320
C24	350
C30	380
C35	400
C40	420